134 3<sup>rd</sup> Ave E Twin Falls, ID 83301 208.736.8050

#### Addendum No. 7

PROJECT: Phase 1 Part A

Wright Ave Jail

Date: September 17, 2024

To the General Contractor, Subcontractors and Suppliers:

The following items contain additions, deletions, or modifications to the Plans and Specifications. This Addendum forms a part of the Contract Documents and shall be bound inside the cover of the Project Manual.

General Contractor shall be responsible for contacting their sub-contractors as this addendum may affect them.

Bidders shall acknowledge receipt of this Addendum on the Contractor Bid Proposal.

#### **GENERAL NOTES/QUESTIONS:**

- 1. Door 62 with HW Set DET07 shall be an Airteq Airglide 8250
- 2. The sewer pipe shall be 6"
- 3. The oil/sand water separator is a 1000 gallon unit
  - a. (1) 2" dia. Vent pipe exiting through the sidewall of the inlet side of the interceptor. Provide and install fittings and materials as needed to have water-tight connection to interceptor.
  - b. (1) 2" dia. Vent pipe exiting through the sidewall of the outlet side of the interceptor. Provide and install fittings and materials as needed to have water-tight connection to interceptor.
  - c. Provide and install 2" dia. Vent pipe between inlet side of interceptor and vent line entering building.
  - d. Provide and install 2" dia. Vent pipe between outlet side of interceptor and vent line entering building.
  - e. Location of 2" dia. Vent pipes entering the building shall be in the Rooftop Access Room 66 behind door 67. Rise (2) vents up above floor 4'-0" and combine into a single 3" VTR.
- 4. Stormwater subsurface seepage bed excavate per plan size, wrap with fabric, fill with 3" minus rock and top with 3//4" gravel Wrap with non-woven geotextile fabric.

#### **STRUCTURAL DRAWINGS**

REVISE Sheet S1.0 per Attached.

#### Summary of Attachments to Addendum No. 7

(Bidders check to verify receipt of all attachments.)

#### **STRUCTURAL DRAWINGS**

Sheet S1.0

END OF ADDENDUM No. 7

### **GENERAL REQUIREMENTS:**

- THE STRUCTURAL SYSTEMS AND MEMBERS DEPICTED HEREIN HAVE BEEN DESIGNED PRIMARILY TO SAFEGUARD AGAINST MAJOR STRUCTURAL DAMAGE AND LOSS OF LIFE, NOT TO LIMIT DAMAGE OR MAINTAIN FUNCTION (IBC SECTION 101.3).
- THESE DRAWINGS, AND THEIR ASSOCIATED STRUCTURAL CALCULATIONS, HAVE BEEN PERFORMED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE STRUCTURAL ENGINEER'S IN THIS OR SIMILAR LOCALITIES. THEY NECESSARILY ASSUME THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKMEN WHO HAVE A WORKING KNOWLEDGE OF THE INTERNATIONAL BUILDING CODE CONVENTIONAL FRAMING REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR FRAMING FLEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, IT IS UNDERSTOOD THAT THE CONTRACTOR WILL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR ALL MISCELLANEOUS WORK NOT
- 3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION SUCH THAT DESIGN LIVE LOAD PER SQUARE FOOT AS STATED HEREIN IS NOT EXCEEDED. OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS USED, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES, AND SHALL COORDINATE ALL DETAILS.
- WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS. THE GREATER REQUIREMENTS SHALL GOVERN. TYPICAL DETAILS AND NOTES ARE NOT NECESSARILY INDICATED ON THE PLANS BUT SHALL APPLY NONE-THE-LESS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT. DETAILS MAY SHOW ONLY ONE SIDE OF CONNECTION OR MAY OMIT INFORMATION FOR
- ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL MECHANICAL, PLUMBING AND ELECTRICAL WITH APPROPRIATE TRADES. DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT AND STRUCTURAL ENGINEER.
- ANY INSPECTIONS, SPECIAL (IBC CHAPTER 17) OR OTHERWISE THAT ARE REQUIRED BY THE BUILDING CODES, LOCAL BUILDING DEPARTMENTS, OR BY THESE PLANS SHALL BE DONE BY AN INDEPENDENT INSPECTION COMPANY OR THE BUILDING DEPARTMENT, SITE VISITS BY THE STRUCTURAL ENGINEER DO NOT CONSTITUTE AN OFFICIAL INSPECTION, UNLESS SPECIFICALLY CONTRACTED FOR.
- SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS, THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL ITEMS NOT IN ACCORDANCE WITH CONTRACT DRAWINGS SHALL BE FLAGGED UPON HIS REVIEW. VERIFY ALL DIMENSIONS WITH ARCHITECT. ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM ORIGINAL CONTRACT DRAWINGS SHALL BE CLOUDED. ANY OF THE AFOREMENTIONED WHICH ARE NOT CLOUDED OR FLAGGED BY SUBMITTING PARTIES. SHALL NOT BE CONSIDERED APPROVED AFTER THE STRUCTURAL ENGINEER'S REVIEW, UNLESS NOTED ACCORDINGLY. ANY ENGINEERING PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A STRUCTURAL ENGINEER REGISTERED IN THE APPROPRIATE STATE. THE SHOP DRAWINGS DO NOT REPLACE THE ORIGINAL CONTRACT DRAWINGS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER ARE NOT TO BE CONSIDERED CHANGES TO ORIGINAL DRAWINGS. THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY THE OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY. REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR. ALLOW (5) WORKING DAYS FOR THE STRUCTURAL ENGINEER'S REVIEW. ONE COPY OF EACH SUBMITTAL WILL BE RETAINED FOR THE STRUCTURAL ENGINEER'S RECORDS.

#### **BASIS FOR DESIGN:**

BUILDING CODE: 2018 EDITION OF THE IBC WITH CITY/COUNTY AMENDMENTS. RISK CATEGORY = IV

### 2. VERTICAL LOADS:

LOCATION	LIVE / SNOW LOAD	DEAD LOAD
ROOF	ROOF = 30 PSF GROUND = 15 PSF	20 PSF
STAIRS	100 PSF	50 PSF

3.	DEFLECTION LIMITS:
Г	ELEMENTS

ANALYSIS PROCEDURE

ELEMENTS	LIVE LOAD	TOTAL LOAD	
ROOF TRUSSES/JOISTS	L/360	L/240	
BEAMS	L/360	L/240	

**EQUIVALENT LATERAL FORCE** 

**PROCEDURE** 

### 4. SEISMIC DESIGN PARAMETERS:

IMPORTANCE FACTOR	le = 1.50	
SITE CLASS	D (ASSUMED)	
SEISMIC DESIGN CATEGORY	С	
MAPPED SPECTRAL RESPONSE ACCELERATIONS	$S_1 = 0.082, S_S = 0.194$	
DESIGN SPECTRAL RESPONSE ACCELERATIONS	$S_{D1} = 0.131, S_{DS} = 0.207$	
PERCENT SNOW INCLUDED WITH SEISMIC LOADS	20	
VERTICAL SHEAR TRANSFER ELEMENTS:		
ORDINARY REINFORCED MASONRY WALLS	R = 2, C <sub>S</sub> = 0.155	
5. WIND DESIGN PARAMETERS (STRENGTH)	:	
ULTIMATE WIND SPEED	113 MPH (3 SECOND GUST)	
WIND EXPOSURE	С	
IMPORTANCE FACTOR	lw = 1.00	
INTERNAL PRESSURE COEFFICIENT	-0.18	
COMPONENT AND CLADDING PRESSURE	27.1 PSF	
NET UPLIFT ON ROOF	21.3 PSF	

### **GENERAL STRUCTURAL NOTES**

1. MINIMUM 28 DAY CONCRETE STRENGTH SHALL BE AS FOLLOWS:

CONCRETE

STRENGTH:

3500 PSI

4500 PSI

4500 PSI AT SALLY POF

CHAPTER 12 OF ACI 318 OR LAP SCHEDULE ON THESE DRAWINGS.

"CLEAR" OR "CLR" ARE TO CENTER OF STEEL. MINIMUM COVER FOR

NON-PRESTRESSED CONCRETE REINFORCING SHALL BE AS FOLLOWS:

MAXIMUM SLUMP FOR ALL CONCRETE SHALL BE 6". PORTLAND CEMENT SHALL

CONFORM TO ASTM C150. TYPE V CEMENT SHALL BE USED FOR CONCRETE IN

NO MORE THAN 90 MINUTES SHALL ELAPSE BETWEEN CONCRETE BATCHING AND

CONCRETE PLACEMENT AND QUALITY SHALL BE PER RECOMMENDATIONS IN ACI

EXCEPT THAT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND AND UNDER

FLOOR DUCTS, ETC. CAST CLOSURE POUR, WHERE SHOWN ON PLANS AROUND

COLUMNS AFTER COLUMN DEAD LOAD IS APPLIED. REMOVE ALL DEBRIS FROM

ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCING, DOWELS, BOLTS,

ALL CONCRETE SLABS ON GRADE SHALL BE DIVIDED INTO AREAS BY CONTROL

JOINTS (KEYED OR SAW CUT) SUCH THAT ONE SLAB AREA DOES NOT EXCEED A

(EXAMPLE: 4" SLAB = 12'-0" LENGTH). SQUARE LAYOUTS ARE PREFERRED, BUT THE

DISTANCE SHALL NOT EXCEED 1.3. IT IS RECOMMENDED THAT SAW CUTS BE MADE

SLAB GEOMETRY MAY DICTATE OTHERWISE. THE RATIO OF THE LONG TO SHORT

KEYED CONTROL JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING

HORIZONTAL PIPES AND ELECTRICAL CONDUITS SHALL NOT BE EMBEDDED IN

10. FLY ASH MAY BE USED ONLY IF PERMITTED BY ARCHITECTURAL SPECIFICATIONS

11. COLD/HOT WEATHER CONCRETE CONSTRUCTION: PROTECT CONCRETE FROM

DAMAGE OR REDUCED STRENGTH IN COMPLIANCE WITH ACI 305 AND 306.

13. LIMIT ALKALI-SILICA REACTION (ASR) TO 0.1% EXPANSION AT 28 DAYS IN CONCRETE

MIX AT ALL EXTERIOR CONCRETE AND INTERIOR CONCRETE EXPOSED TO

12. CONCRETE MIXES SHALL BE DESIGNED BY A CERTIFIED LABORATORY AND

AND SHALL BE LIMITED TO 18 PERCENT OF CEMENTITIOUS MATERIALS AND SHALL

HAVE A REPLACEMENT FACTOR OF 1.2 RELATIVE TO CEMENT REPLACED. NO FLY

ASH ADDITIVES SHALL BE USED IN FLATWORK OR ARCHITECTURALLY EXPOSED

STRUCTURAL CONCRETE AND SLABS ON GRADE EXCEPT WHERE SPECIFICALLY APPROVED OR NOTED BY THE STRUCTURAL ENGINEER. PIPES AND CONDUITS

MAXIMUM LENGTH OF 36 TIMES THE SLAB THICKNESS IN BOTH DIRECTIONS

ANCHORS, PIPES, SLEEVES, ETC., SHALL BE SECURELY POSITIONED IN THE FORMS

614, ACI 301 AND ACI 318. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED,

CONCRETE PLACEMENT UNLESS APPROVED BY THE TESTING AGENCY.

3500 PSI ELSEWHERE

ALL NORMAL WEIGHT CONCRETE SHALL BE REGULAR WEIGHT OF 150 POUNDS PER

REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE

CORNERS AND INTERSECTIONS PER TYPICAL DETAILS. VERTICAL WALL BARS SHALL

ALL DIMENSIONS SHOWING THE LOCATION OF REINFORCING STEEL NOT NOTED AS

CUBIC FOOT USING HARD ROCK AGGREGATES. AGGREGATE USED IN CONCRETE

LAP SPLICES FOR BEAMS AND FLOOR SLABS SLABS SHALL BE ACCORDING TO

STAGGER SPLICES A MINIMUM OF ONE LAP LENGTH. NO TACK WELDING OF

STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT ALL

**CONCRETE:** 

FOUNDATION WALLS

INTERIOR CONCRETE

SHALL CONFORM TO ASTM C33.

BE SPLICED AT OR NEAR FLOOR LINES.

CAST AGAINST EARTH (FOOTINGS)

STRUCTURAL SLABS AND WALLS

TIES, STIRRUPS AND SPIRALS

SLABS ON GRADE

GROUND ROOF SLAB

LOCATION:

EXPOSED TO EARTH OR WEATHER - #5 AND SMALLER

EXPOSED TO EARTH OR WEATHER - #6 AND LARGER

NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE

BEAMS AND COLUMNS (PRIMARY) REINFORCEMENT,

FORMS BEFORE PLACING CONCRETE.

BEFORE PLACING THE CONCRETE.

ITHIN 16 HOURS OF CONCRETE BATCHING.

POURING, ALL OTHER JOINTS MAY BE SAW CUT.

SHALL NOT IMPAIR THE STRENGTH OF THE WORK.

APPROVED BY THE STRUCTURAL ENGINEER.

CONTACT WITH ALKALINE SOIL, AND TYPE II ELSEWHERE.

SLABS ON GRADE

USE:

**FOOTINGS** 

(APPLY UNLESS NOTED OTHERWISE ON PLANS/DETAILS)

MAX W/C

RATIO

0.50

0.45

MINIMUM

COVER

1 1/2"

1 1/2"

3/4"

1 1/2"

AIR

**ENTRAINMENT** 

5.5% ± 1%

5.5% ± 1%

**TOLERANCE** 

± 3/8"

± 1/4"

± 3/8"

± 3/8"

1/8"

1/8"

3/8"

### **FOUNDATION NOTES:**

FOUNDATIONS HAVE BEEN DESIGNED IN CONFORMANCE WITH RECOMMENDATIONS BY: STRATA REPORT NO. TF24135E DATED SEPTEMBER 6,

SITE PREPARATION AND GRADING REQUIREMENTS OF THE SOIL REPORT AND ANY ADDENDUMS SHALL BE COMPLETED PRIOR TO CONSTRUCTION OF FOUNDATIONS. ANY TESTS OR INSPECTIONS REQUIRED BY THE SOIL REPORT SHALL BE PERFORMED PRIOR TO PLACEMENT OF FOUNDATION REINFORCING STEEL OR CONCRETE. ALTERATIONS TO SITE PREPARATION OR GRADING SHALL BE REPORTED TO THE GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION CONSTRUCTION.

#### THE SOIL DESIGN VALUES FOR THE FOUNDATION ARE:

7	ALLOWABLE BEARING PRESSURE	3000 PSF
}	ALLOWABLE LATERAL BEARING PRESSURE	300 PSF/FT
	ALLOWABLE LATERAL SLIDING COEFFICIENT	0.35
\ \ \	LATERAL BACKFILL PRESSURE (UNRESTRAINED)	37 PSF/FT
\ \ \	LATERAL BACKFILL PRESSURE (RESTRAINED)	62 PSF/FT

3. A ONE-THIRD INCREASE IN BEARING PRESSURES IS ALLOWED WITH SEISMIC OR WIND LOAD COMBINATIONS. LATERAL BEARING AND LATERAL SLIDING RESISTANCE MAY BE COMBINED.

FOUNDATION BEARING DEPTH	
24" BELOW FINISHED GRADE	

ALL FOUNDATIONS SHALL BEAR ON A MIN. OF 3'-0" OF OVER-EXCAVATED AND RE-COMPACTED ENGINEERED FILL OR COMPETENT NATIVE SOIL SUBBASE (EXISTING ON-SITE LOESS IS ACCEPTABLE) COMPACTED TO 95% DRY DENSITY (MODIFIED PROCTOR). ALL OVER-EXCAVATIONS SHOULD EXTEND 1 FOOT LATERALLY BEYOND THE IMPROVEMENT AREA FOR EVERY 2 FEET OF EXCAVATION DEPTH. CONCRETE FOOTING EXCAVATIONS SHALL BE CLEAN AND FREE OF LOOSE DEBRIS OR UN-COMPACTED MATERIAL AT TIME OF CONCRETE PLACEMENT.

- GRADE IS DEFINED AS LOWEST ADJACENT GRADE WITHIN 5 FEET OF THE BUILDING FOR PERIMETER FOOTINGS. WHERE EXTERIOR PAVING OR CONCRETE IS DIRECTLY ADJACENT TO BUILDING, GRADE IS DEFINED AS TOP OF EXTERIOR PAVING AT LEAST 5 FEET FROM BUILDING.
- CONCRETE SLABS ON GRADE SHALL BE SUPPORTED ON A 6 INCH (MIN) LAYER OF FREE-DRAINING GRANULAR MAT (DRAINAGE FILL COURSE). THE MAT SHOULD CONSIST OF A WELL GRADED SAND AND GRAVEL MIXTURE WITH MAXIMUM 3/4-INCH CRUSHED AGGREGATE. THE GRANULAR MAT SHOULD BE COMPACTED TO NO LESS THAN 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557 PROVIDE A VAPOR BARRIER WHERE REQUIRED PER ARCHITECTURAL DRAWINGS.

#### REINFORCING STEEL:

ASTM A615 GRADE 60 (FY = 60 KSI) DEFORMED BARS FOR ALL BARS #4 AND LARGER. ASTM A615 GRADE 40 (FY = 40 KSI) DEFORMED BARS FOR ALL BARS #3 AND SMALLER. GRADE 60 DEFORMED BARS SHALL BE USED FOR CONCRETE WALLS, BEAMS, AND ELEVATED SLAB REINFORCING.

- WELDING OF REINFORCING BARS SHALL BE MADE ONLY TO ASTM A706 GRADE 60 BARS AND ONLY USING E90 SERIES RODS. WELDING OF REINFORCING BARS SHALL BE MADE ONLY AT LOCATIONS SHOWN ON PLANS OR DETAILS.
- REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE.

### STEEL:

- MATERIALS: ROLLED W SHAPES. SHALL CONFORM TO ASTM A992 (FY=50 KSI). ALL OTHER STRUCTURAL STEEL SHAPES, ROLLED SECTIONS, BARS AND PLATES SHALL CONFORM TO ASTM A36 (FY = 36 KSI). ALL PIPE STEEL SHALL BE ASTM A501 (FY = 36 KSI) OR ASTM A53, TYPE E OR S, GRADE B (FY = 35 KSI). ALL TUBULAR STEEL SHALL BE ASTM A500 GRADE C (FY = 50 KSI).
- ALL BOLTS AND STUDS SHALL BE ASTM A307, UNLESS NOTED OTHERWISE. ALL EXPANSION BOLTS TO HAVE CURRENT ICC REPORT RATING FOR MATERIAL INTO WHICH INSTALLATION TAKES PLACE. HEADED STUDS SHALL CONFORM TO ALL REQUIREMENTS OF THE LATEST EDITION OF THE "RECOMMENDED PRACTICES FOR STUD WELDING" AND THE "STRUCTURAL WELDING CODE" PUBLISHED BY AWS. ALL BOLTS, ANCHOR BOLTS, EXPANSION BOLTS, ETC. SHALL BE INSTALLED WITH STEEL WASHERS AT FACE OF WOOD OR AT SLOTTED HOLES IN STEEL SECTIONS.
- ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS FOR THE DESIGN. FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, LATEST
- WELDING SHALL BE BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. ALL WELDING SHALL USE E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. ALL WELDING PER LATEST AMERICAN WELDING SOCIETY STANDARDS. ALL WELDS ON DRAWINGS ARE SHOWN AS SHOP WELDS. CONTRACTOR MAY SHOP WELD OR FIELD WELD AT HIS DISCRETION. ALL FULL PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING LABORATORY.
- 5. STEEL TO STEEL BOLTED CONNECTIONS: HIGH STRENGTH BOLTS SHALL BE ASTM A325N AND SHALL BE INSTALLED AS BEARING-TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE (TYPE "N" CONNECTION UNLESS NOTED OTHERWISE). BOLTS MAY BE TIGHTENED USING ANY AISC APPROVED METHOD.
- DRYPACK SHALL BE 5,000 PSI FIVE STAR NON-SHRINK GROUT OR EQUIVALENT. INSTALL DRYPACK UNDER BEARING PLATES BEFORE FRAMING MEMBER IS INSTALLED. AT COLUMNS. INSTALL DRYPACK UNDER BASE PLATES AFTER COLUMN HAS BEEN PLUMBED BUT PRIOR TO FLOOR OR ROOF INSTALLATION.

## WOOD:

- GENERAL: DO NOT NOTCH OR DRILL JOISTS, BEAMS, OR LOAD BEARING STUDS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER THROUGH THE ARCHITECT. DOUBLE UP JOISTS AND BLOCKING UNDER PARTITIONS. PROVIDE 2" (NOMINAL) SOLID BLOCKING AT SUPPORTS OF ALL JOISTS. UNLESS NOTED OTHERWISE ON PLANS/DETAILS PROVIDE 2x SOLID BLOCKING AT MID-HEIGHT OF BEARING STUD WALLS, ALL NAILING NOT NOTED SHALL BE ACCORDING TO IBC TABLE 2304.10.1. JOIST HANGERS AND OTHER MISC. FRAMING ANCHORS SHALL BI SIMPSON STRONG-TIE COMPANY, INC. OR OTHER MANUFACTURER WITH CURRENT ICC-ES APPROVAL.
- SAWN LUMBER: FRAMING LUMBER SHALL COMPLY WITH THE LATEST EDITION OF THE GRADING RULES OF THE WESTERN WOOD PRODUCTS ASSOCIATION (WWPA) OR THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB), ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED LUMBER GRADING AGENCY. SAWN LUMBER SHALL HAVE THE FOLLOWING MINIMUM GRADE UNLESS NOTED OTHERWISE IN SCHEDULES:

USE:	MATERIAL:	
2x4 STUDS	DOUGLAS-FIR NO. 2, MINIMUM (U.N.O.)	
2x6 STUDS	DOUGLAS-FIR NO. 2, MINIMUM (U.N.O.)	
JOISTS, TOP PLATES AND ALL OTHER SAWN LUMBER	DOUGLAS-FIR NO. 2, MINIMUM (U.N.O.)	
BEAMS AND POSTS	DOUGLAS-FIR NO. 2, MINIMUM (U.N.O.)	

3. PLYWOOD: ALL PLYWOOD SHALL BE C-D OR C-C SHEATHING CONFORMING TO STANDARD PS 1-95, LAY UP PLYWOOD WITH FACE GRAIN IN PERPENDICULAR TO SUPPORTS (ON ROOFS WHERE PLYWOOD IS LAID UP WITH FACE GRAIN PARALLEL TO SUPPORTS, USE A MINIMUM OF 5-PLY PLYWOOD, STAGGER JOINTS). ALL NAILING, COMMON NAILS. BLOCKING AT PANEL EDGES WHERE INDICATED ON PLANS. ALL PLYWOOD SHALL BE OF THE FOLLOWING NOMINAL THICKNESS, SPAN/INDEX RATING

AND SHALL BE NAILED AS FOLLOWS UNLESS NOTED OTHERWISE ON THE PLANS:					
	LOCATION:	NOMINAL THICKNESS:	SPAN INDEX RATING:	EDGE ATTACHMENT:	FIELD Attachment:
	WALL	7/16" OR 1/2"	24/16	8d AT 6" O.C.	8d AT 12" O.C.
	ROOF	7/16" OR 1/2"	24/16	8d AT 6" O.C.	8d AT 12" O.C.
	ROOF	15/32" OR 1/2"	32/16	8d AT 6" O.C.	8d AT 12" O.C.
	ROOF	19/32" OR 5/8"	40/20	10d AT 6" O.C.	10d AT 12" O.C.
	ROOF	23/32" OR 3/4"	48/24	10d AT 6" O.C.	10d AT 12" O.C.
	ROOF	7/8"	60/32	10d AT 6" O.C.	10d AT 12" O.C.
	FLOOR	3/4" T&G	48/24	10d AT 6" O.C. OR #8 SCREWS AT 6" O.C.	10d AT 6" O.C. OR #8 SCREWS AT 12" O.C
	FLOOR	7/8" T&G	60/32	10d AT 6" O.C. OR #8 SCREWS AT 6" O.C.	10d AT 6" O.C. OR #8 SCREWS AT 12" O.C
	FLOOR	1 1/8" T&G	60/48	10d AT 6" O.C. OR	10d AT 6" O.C. OR

SCREWS AT FLOOR SHEATHING SHALL BE #8 SCREWS AND SHALL PENETRATE AT LEAST 1 1/2" INTO THE SUPPORTING MEMBER. ALL FLOOR SHEATHING SHALL BE GLUED TO SUPPORTING MEMBERS WITH AN APA AFG-01 QUALIFIED ADHESIVE.

#8 SCREWS AT 6" O.C. #8 SCREWS AT 12" O.C.

PLYWOOD ALTERNATE: AMERICAN PLYWOOD ASSOCIATION PERFORMANCE RATED SHEATHING MAY BE USED AS AN ALTERNATE TO PLYWOOD WITH PRIOR APPROVAL OF OWNER, ARCHITECT AND ROOFER. IT MAY NOT BEUSED ON ROOFS WHERE BUILT UP ROOF SYSTEM IS TO BE GUARANTEED BY ROOFER. RATED SHEATHING SHALL COMPLY WITH CURRENT ICC-ES REPORTS AND SHALL HAVE A SPAN RATING EQUIVALENT TO OR BETTER THAN THE PLYWOOD IT REPLACES. ATTACHMENT AND THICKNESS (WITHIN 1/32") SHALL BE THE SAME AS THE PLYWOOD IT REPLACES. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.

- NOMINAL 2x AND 3x DECKING. TONGUE AND GROOVE TYPE. MINIMUM Fb = 1,600 PS MINIMUM E = 1,300,000 PSI. INSTALL WITH TONGUES UP SLOPE ON PITCHED ROOFS, AND OUTWARD IN THE DIRECTION OF LAYING ON FLAT ROOFS. NAIL EACH PLANK WITH 16d TOENAIL (THRU THE TONGUE) AND 16d FACE NAIL AT EACH SUPPORT. DECK SHALL BE INSTALLED AS SIMPLE SPAN WITH ALL PLANKS BEARING ON TWO SUPPORTS. FOR REFERENCE AND/OR ADDITIONAL INFORMATION SEE AITC 117-2010.
- 5. GLUED-LAMINATED BEAMS (GLB): GLUED-LAMINATED BEAMS SHALL BE DOUGLAS FIR COMBINATION AT 24F-V4 AT SIMPLE SPAN BEAMS AND 24F-V8 AT MULTI-SPAN AND CANTILEVERED BEAMS WITH THE FOLLOWING MINIMUM PROPERTIES: FB = 2,400 PSI FV = 190 PSI, FC (PERPENDICULAR) = 650 PSI, E =1.800 KSI, ALL BEAMS SHALL BE FABRICATED USING WATERPROOF GLUE, FABRICATION AND HANDLING PER LATEST AITC AND WCLA STANDARDS. BEAMS TO BEAR GRADE STAMP AND AITC STAMP AND CERTIFICATE. CAMBER AS SHOWN ON DRAWINGS. STANDARD CAMBER IS BASED ON A RADIUS OF CURVATURE OF 2000 FEET.
- GLUED-LAMINATED COLUMNS: GLUED-LAMINATED COLUMNS SHALL BE DOUGLAS FIR COMBINATION 3 WITH THE FOLLOWING MINIMUM PROPERTIES: FBY = 2,100 PSI, FBX = 2000 PSI, FVY = 230 PSI, FVX = 265 PSI, FC (PERPENDICULAR) = 650 PSI, E = 1,900 KSI. ALL COLUMNS SHALL BE FABRICATED USING WATERPROOF GLUE. FABRICATION AND HANDLING PER LATEST AITC AND WCLA STANDARDS. COLUMNS TO BEAR GRADE STAMP AND AITC STAMP AND CERTIFICATE.
- LAMINATED VENEER LUMBER (LVL): DESIGN, FABRICATION AND ERECTION IN ACCORDANCE WITH THE LATEST ICC-ES REPORT. MINIMUM PROPERTIES FOR LVLs SHALL BE: FB = 2,600 PSI, FV = 285 PSI, E = 2,000 KSI.
- PARALLEL STRAND LUMBER (PSL): DESIGN, FABRICATION AND ERECTION IN ACCORDANCE WITH THE LATEST ICC-ES REPORT. MINIMUM PROPERTIES FOR PSLs SHALL BE: FB = 2,900 PSI, FV = 290 PSI, E = 2,000 KSI.
- 9. LAMINATED STRAND LUMBER (LSL): DESIGN, FABRICATION AND ERECTION IN ACCORDANCE WITH THE LATEST ICC-ES REPORT. MINIMUM PROPERTIES FOR LSLs SHALL BE: FB = 2,325 PSI, FV = 310 PSI, E = 1,550 KSI.
- 10. SILL PLATES RESTING ON CONCRETE OR MASONRY SHALL BE OF TREATED FIR. SHEAR WALLS AND EXTERIOR WALL SILLS AT CONCRETE SLAB SHALL HAVE A MINIMUM OF (2) ANCHOR BOLTS PER PIECE. PROVIDE ANCHOR BOLT AT 9" MAXIMUM, 4" MINIMUM FROM THE END OF EACH PIECE AT SPLICE OR END OF WALL. MAXIMUM ANCHOR BOLT SPACING SHALL BE 72" ON CENTER UNLESS NOTED OTHERWISE ON PLANS OR DETAILS. ALL ANCHOR BOLTS (OTHER THAN BOLTS FOR HOLDOWNS) SHALL EMBED 7" INTO CONCRETE. ANCHOR BOLTS FOR HOLDOWNS SHALL NOT BE CONSIDERED AS PART OF REQUIRED ANCHOR BOLTS ON SHEAR WALLS. ALL EXTERIOR WALLS SHALL BE SECURED WITH MINIMUM ANCHOR BOLTS. INTERIOR WALLS MAY BE SECURED TO CONCRETE WITH EITHER ANCHOR BOLTS OR POWER DRIVEN SHOT PINS UNLESS NOTED OTHERWISE ON PLANS.
- 11. BOLTING: ALL BOLTS IN WOOD CONNECTIONS SHALL CONFORM TO ASTM A307. BOLTS SHALL BE INSTALLED IN HOLES BORED WITH A BIT 1/16" LARGER THAN THE Ø (DIAMETER) OF THE BOLT. BOLTS AND NUTS SEATING ON WOOD SHALL HAVE CUT STEEL WASHERS UNDER HEADS AND NUTS. NICK THREADS TO PREVENT LOOSENING.
- 12. PREFABRICATED WOOD TRUSSES: PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED TO SUPPORT SELF WEIGHT PLUS LIVE LOAD AND SUPERIMPOSED DEAD LOADS. WHERE UNINHABITABLE ATTIC SPACE CAN BE USED FOR STORAGE, A 20 PSF LIVE LOAD ON THE BOTTOM CHORD SHALL BE INCLUDED IN THE ANALYSIS. BRIDGING SIZE AND SPACING BY TRUSS MANUFACTURER UNLESS NOTED OTHERWISE, CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WITH DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER FOR REVIEW PRIOR TO MANUFACTURE FOR BOTH ROOF AND FLOOR TRUSSES WHEN USED.

SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS, ALL CONNECTORS SHALL HAVE CURRENT ICC-ES APPROVAL, ADDITIONAL TRUSSES SHALL BE SUPPLIED AS REQUIRED TO SUPPORT MECHANICAL EQUIPMENT. PER IBC SECTION 2303.4 AND TPI-1: EACH TRUSS SHALL BE LEGIBLY BRANDED, MARKED OR OTHERWISE HAVE PERMANENTLY AFFIXED THERETO THE IDENTITY OF THE COMPANY MANUFACTURING THE TRUSS, THE DESIGN LOADS, AND THE TRUSS SPACING - WITHIN TWO FEET OF THE CENTER OF THE SPAN ON THE FACE OF THE BOTTOM CHORD.

PREFABRICATED WOOD/STEEL WEB JOIST/PURLINS (TJI/TJL SERIES OR EQUAL): DESIGN, FABRICATION AND ERECTION IN ACCORDANCE WITH THE LATEST EDITION ICC-ES REPORT. CONNECTIONS AND BEARING MATERIAL TO BE DESIGNED AND FURNISHED BY JOIST FABRICATOR, CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WITH DESIGN CALCULATIONS SEALED BY A REGISTERED STRUCTURAL ENGINEER FOR REVIEW PRIOR TO MANUFACTURE. ADDITIONAL JOISTS SHALL BE SUPPLIED AS REQUIRED TO SUPPORT MECHANICAL EQUIPMENT.

### **DEFERRED SUBMITTAL ITEMS:**

**SYMBOLS LEGEND** 

(NOT ALL SYMBOLS NECESSARILY APPLY TO THIS PROJECT)

WALLS WITH SOLID LINES DESIGNATE STRUCTURAL (BEARING) WALLS.

<u>√5</u>, <u>∕6</u>, <u>√7</u> -AS SHOWN ON PLAN INDICATES A SHEARWALL; HATCHING IN WALL

(A). (B). - AS SHOWN ON PLAN INDICATES A SHEARWALL HOLDOWN. SEE HOLDOWN

XXXX CS16, CMSTC16, ECT. - AS SHOWN AT WALL OPENINGS INDICATE STRAPPING,

D=xxx# INDICATES DRAG LOAD (ASD) THAT TRUSS MANUFACTURER IS TO DESIGN TRUSS

PX P1, P2, ETC. AS SHOWN ON PLAN INDICATES A WOOD POST. SEE POST SCHEDULE

SCX SC1, SC2, ETC. - AS SHOWN ON PLAN INDICATES A STEEL COLUMN. SEE STEEL

COLUMN SCHEDULE FOR ADDITIONAL INFORMATION. COLUMNS START AT THE

**ABBREVIATIONS** 

K(KIP)

LBS (#)

W.W.F

GLB (GLULAM) — GLUED-LAMINATED BEAM

1000 POUNDS

LIVE LOAD

- MINIMUM

- INSIDE FACE OF WALL

LONG LEG HORIZONTAL

OUTSIDE FACE OF WALL

PRECAST CONCRETE

SHORT LEG HORIZONTAL

SHORT LEG VERTICAL

LONG LEG VERTICAL

MANUFACTURER('S)

NOT APPLICABLE

NOT TO SCALE

PREFABBICATED

REINFORCING

TOP OF BEAM

TOP OF DECK

TOP OF LEDGER

TOP OF STEE

TOP OF WALL

TYPICAL

- VERTICAL

WITHOLIT

TOP OF MASONR`

UNLESS NOTED OTHERWISE

WELDED WIRE FABRIC

ON CENTER

- OPPOSITE

PROVIDE STRAPPING PER "TYPICAL STRAP AT OPENING" DETAIL.

WALLS WITH HATCH DESIGNATE MASONRY WALLS.

XXX# FOR FRAMING INFORMATION.

FOR IN BOTH TENSION AND COMPRESSION.

FOR MORE INFORMATION

C=X" INDICATES CAMBER IN BEAM.

STEP INDICATES STEPPED OR DEPRESSED SLAB.

LEVEL THEY ARE CALLED OUT ON.

- AGGREGATE BASE COURSE

ABOVE FINISHED FLOOR

BFI OW FINISHED FLOOR

AT (MEASUREMENT)

BOTTOM OF BEAM

BOTTOM OF FOOTING

CENTERLINE OF BEAM

CENTERLINE OF WALL

CENTERLINE OF COLUMN

CENTERLINE OF FOOTING

CONCRETE CONTROL JOIN

CONCRETE SAWCUT JOINT

CONCRETE MASONRY UNIT

AIR CONDITIONER

ALTERNATE

ANCHOR BOLT

CENTERLINE

CONTINUOUS

DIAMETER

- DRAWING(S

EQUIPMENT

- EXISTING

- EACH WAY

- FOLIAI

EXP. JT (E.J.) — EXPANSION JOINT

- EDGE OF SLAB

- EXPANSION BOLT

FINISHED FLOOR

FACE OF MEMBER

- GENERAL STRUCTURAL NOTES

FACE OF STEEL

FACE OF WALL

GALVANIZED

- GAUGE

· CI FAR

C.L.W.-

CONN.

ø OR DIA. -

DWG(S)

F.O.W.-

INDICATES RIDGID CONNECTION.

DESIGNATES SHEARWALL LENGTH

- AS SHOWN ON PLAN INDICATES A HELICAL PIER.

SCHEDULES AND DETAILS FOR ADDITIONAL INFORMATION.

WALLS WITH DASHED LINES DESIGNATE NON-STRUCTURAL (NON-BEARING) WALLS.

INDICATES HVAC EQUIPMENT ON ROOF OR IN ATTIC SPACE. SEE TYPICAL DETAILS

PREFABRICATED STEEL JOISTS



09/12/2024

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# MASONRY CONTROL JOINT POUNDS PER LINEAR FOOT POUNDS PER SQUARE FOO POUNDS PER SQUARE INCH O **(1)** it ch plannii 11c Idaho 833

S

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a

 $\dot{\mathbf{C}}$ 

	SHEET INDEX	
SHEET	DESCRIPTION	DETAILS
S1.0	GENERAL STRUCTURAL NOTES	
S1.1	GENERAL STRUCTURAL NOTES	
S1.2	TYPICAL DETAILS	T - SERIES
S1.3	TYPICAL DETAILS	T - SERIES
S1.4	TYPICAL DETAILS	T - SERIES
S2.0	FOUNDATION PLAN	
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S3.0	FOUNDATION DETAILS	100 - SERIES
S4.0	FRAMING DETAILS	200 - SERIES
S4.1	FRAMING DETAILS	200 - SERIES

JOB NO.: 24.145 PROJECT MANAGER: JJ CAD OPERATOR: GTC

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DATE: 9.12.2024

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